

Kurdistan Region Government Ministry of Higher Education and Scientific Research Erbil Polytechnic University



Module (Course Syllabus) Catalogue

2022-2023

College/ InstituteErbil Polytechnic UniversityDepartmentHighway Engineering Technique DepartmentModule NameFoundations Analysis & DesignModule CodeFAD704DegreeTechnical DiplomaBachelor ✓ High DiplomaMasterPhDSemester7thQualificationM.Sc. Structural Engineering Scientific TitleScreetistECTS (Credits)6
Department Module Name Foundations Analysis & Design Module Code FAD704 Degree Technical Diploma Bachelor √ High Diploma Master PhD Semester 7 th Qualification M.Sc. Structural Engineering Scientific Title Assistant Lecturer ECTS (Credits) 6
Module NameFoundations Analysis & DesignModule CodeFAD704DegreeTechnical DiplomaBachelor √High DiplomaMasterPhDSemester7thQualificationM.Sc. Structural EngineeringScientific TitleAssistant LecturerECTS (Credits)6
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Module type Prerequisite Core 🗸 Assist.
Weekly hours 4 hours
Weekly hours (Theory) (4) hr Class (108) Total hrs Workload
Weekly hours (Practical) (None)hr Class (None)Total hrs Workloa
Number of Weeks 15
Lecturer (Theory) Ali J. Nouri Al – Barazanchi
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Lecturer (Practical) None
E-Mail & Mobile NO.
Websites

Course Book

Course Description	Increase student knowledge and learn the principles and practices for the investigation, design, contracting, and construction of shallow, intermediate, and deep foundations, including remediation of soft, wet, expansive, and frost-prone soils. After attending this course, student shall have a firm grasp of the background and design specifics necessary to compete in this industry, including industry-leading information on the principles and practices of foundation design for buildings, transportation infrastructure, utilities, and industrial facilities. Understand practical emerging technologies including advanced design techniques for pressuremeter-supported foundation design; aggregate piers; auger cast, helical, and micro piles; and design for lateral loads, frost heave, and wet/dry cycles.	
Course objectives	 Understand the importance of geotechnical investigation in foundation design Apply analytical skills to solving problems in foundation design Understand the basic requirements of international codes for foundation design Appreciate the interaction between soils and structures Identify the key geotechnical and structural issues in foundation design Appreciate the range of foundation types available and their application Select an appropriate foundation system for a structure Appreciate the practical problems of design and detailing when designing foundations Introduce the student to certain case studies 	
Student's obligation	 a. To attend the classes regularly with minimum absence. b. To participate actively in the class discussion and Q&A session c. Study on daily basis to digest the class material d. To write note off-handouts e. Prepared for sudden Quizzes f. Vet through the references provided by the lecturer and to solve as much as possible of homework and exercises for the subjective materials. g. Prepare the assignment and the seminar as instructed by the lecturer. 	
Required Learning Materials	Students at this stage with the workload assigned technical for the subject are not required to scatter their attention with bunch of sources. Students are encouraged to thoroughly study the refence given by the lecturer and to vet through available cyber data related to the subject and this shall include the concrete technology worked examples and all those are support with construction site visit for the students to appreciate and monitor closely the application of the theoretical concept in construction.	

	Task		Weight (Marks)	Due Week	Relevant Learning Outcome	
	Paper Review			None for B.SC.		
		Homework	10	Weekly	Application for subject by subject	
		Class Activity	2	Weekly	Participate in syllabus learning	
	Assignments	Report	8	4 th & 8 th	Concentrate on certain subject of the module and cover its technical aspects	
Evaluation	nents	Seminar	8	6 th & 10 th	Individual or in group for subjects within the module but out of the syllabus	
		Essay				
		Project				
	Quiz		8			
	Lab.			46		
	Midterm Exam		24	7 th		
	Final E	xam	40	14 th & 15 th		
	Total	ad of the current	100	studant shall	ha abla ta laarn tha majar	
Specific learning outcome:	By the end of the current course, the student shall be able to learn the major activities related to the foundation analysis and design which is the part the makes the backbone for any constructional project. The student would be able to put a scenario for the soil investigations works, assess the subsoil bearing capacity, decide on the proper foundation for the structure, calculate the anticipated settlement, design the foundations (concrete wise for shallow foundations (Spread, Continuous, Strip and Raft) and deep (Piled) foundations, soil treatment for strengthening and retaining structure. The most effect matter the student learn in this course is to decide on safe and most economical foundation system for the subjective projects.					
	Foundation	ation Analysis and	d Design: Jose	ph E. Bowles		
	 Principles of Foundation Engineering: Braja M. Das 					
	 Shallow foundations bearing capacity and settlement: Braja M. Das 				nt: Braia M. Das	
6			• • •	hnical Engineering: Braja M. Das		
Course	 Foundation Design: Principles and Practices (3rd Edition) 3rd Edition by Donald 					
References:	P. Cod	uto (Author) & W				

Course topics (Theory)	Weeks	Learning Outcome	
Subsoil Explorations			
1. Introduction	1 st	Under this subject the	
2. Soil Explorations Scenario	1 st	student shall be	
3. Number of boreholes and test pits	1 st	introduced to the importance of the	
4. Depth of boreholes and type of drillings	1 st	importance of the explorations to identify	
5. Sampling and types of samples	2 nd	the soil characteristic	
6. Laboratory tests over soil samples	2 nd	which shall be used as the	
7. Field tests	2 nd	based for the bearing capacity of the subsoil and	
8. Outcome of tests	2 nd	evaluate the capacity of	
9. Reporting	2 nd	deep pile	
Bearing Capacity of Shallow Foundations			
1. Introduction	3 rd		
2. Bearing Capacity equations for Shallow Found	lations 3 rd	Student shall learn the	
3. Effect of water table over the bearing capacit	y 3 rd	evaluation of the bearing capacity for shallow	
4. Factor influencing Bearing Capacity	3 rd	foundations from shear	
5. Bearing capacity for eccentrically loaded foun	dations 4 th	strength parameters and	
6. Layered soil bearing capacity	4 th	field tests for all types of shallow foundations (Spread, Strip & Raft)	
7. Bearing capacity form field tests – SPT	4 th		
8. Bearing Capacity from field test - PLT	4 th		
Settlements of Foundations			
1. Introduction	5 th	Students shall learn under	
2. Types of Settlements	5 th	the chapter's syllabus the difference between	
3. Short Term Settlements (Immediate Settleme	ents) 5 th	_ immediate and	
4. Consolidation Settlements	6 th	consolidation settlement	
5. Time – Consolidation scenario	6 th	and how to calculate each	
6. Solved examples	6 th	with time – consolidation scenario	
Deep (Pile) Foundations			
1. Introduction	7 th	Student shall learn by end	
2. Types of Piles	7 th	of this chapter when he	
3. Analysis of Piles Capacity	7 th	should decide to go for	
4. Pile End Bearing Evaluation	7 th	deep foundation and the analysis of the pile's	
5. Pile Skin Friction Evaluation	8 th	capacity and ultimately	
6. Calculate Pile Capacity	8 th	the design of the number	

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7. Group of Piles	8 th	of piles required under		
8. Block Shear of Piles	8 th	load column with its pile		
9. Piles under eccentric load	8 th	cap. The student shall also learn the testing methods		
10. Settlement of piles	9 th	for piles to ensure their		
11. Case of broken piles	9 th	carrying capacity.		
12. Estimate pile length of designated load	9 th			
13. Design of piles with pile caps	9 th			
14. Testing of Piles	9 th			
Reinforced Concrete Shallow Foundation Design				
1. Introduction	10 th	Student shall learn in this		
2. Types of shallow foundations	10 th	chapter to differentiate between the different types of the shall ow foundations. The		
3. Analysis and design of spread foundations	10 th			
4. Analysis and design of strip foundations	11 th			
5. Analysis and design of combined footing	11 th	reinforced concrete design shall be taught for spread, strip, combined		
6. Analysis and design of trapezoidal combined footing	11 th			
7. Analysis and design of raft foundations	12 th & 13 th	and raft foundation after ACI – 19 Code of Practice		
Questions Example Design Attached copy for the academic year 2021 – 2022, final, first Attempt exam with solution Extra notes: None so far External Evaluator				

Ministry of Higher Education & Scientific Research Erbil Polytechnic University Erbil Technical Engineering College	EPU	Class: 4 th year Subject: Foundations Design Time: 3Hrs. Date: May 19 ^{th,} 2022
Highway Engineering Techniques Department Exam is Open Notes Only	2021 – 2022 Final Examination – 1 st Attempt	Code: HE406

Answer all Questions – All questions hold same marks (25 Marks)

Q.1) The results of two plate load tests are tabulated below:

Plate Size (Circular 0.3m)		Plate Size (Circular 0.6m)	
Pressures (KPa)	Settlement (mm)	Pressures (KPa)	Settlement (mm)
150	8	50	3
300	14	100	9
450	23	150	17
600	30	200	21
750	40	250	24
900	52	300	30

A square column foundation has to be constructed to carry a total load of 850KN with tolerable settlement of 30mm. Determine the size of the foundation using **Housel Method**.

Q.2) The Administrative building for a Highway's Toll Plaza has a raft foundation with the dimensions of (15 X 25m) subjected to a net load of (93750kN) is to be constructed over a soil consists of three basic layers. The top (first) layer is Clay and the second layer is Sand resting over the third layer which is Clay again. Both clay layers are over consolidated with ($\sigma'_o + \Delta \sigma'_{ave.} < \sigma'_c$). The foundation is at depth of 1.5m below NGL which is the same depth of the ground water table. As it is required to calculate the Primary Consolidation settlement only, make use of the following data and estimate the total settlement using Stress Influence Tables:

a) Thickness of top first Clay Layer = 4.5m, **b**) Thickness of the second Sand Layer is 3m **c**) Thickness of the third clay layer is 9m **d**) Sand & Clays total density = 18kN/m³ & saturated density = 20kN/m³ **e**) C_s = 0.040, e_o = 0.72 for both encountered Clay layers. (Sketch the layers for easy understanding)

Q.3) Design a pile foundation (Number of piles and the dimension of the pile cap) to support a working load of 21000KN. The pile shall pass through the following layers:

Layer No.	Thickness of Layer, (m)	Type of Soil	C (kPa)	Ø (°)	Ytot. (kN/m ³)
1	5m	Soft Clay	30	0	18
2	8m	Medium Sand	0	20	18
3	7m	Dense Sand	0	30	18
4	10m	Stiff Clay	100	0	18

Consider the following:

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- 1. Piles are Precast Driven Square 400mm X 400mm in size
- 2. No ground water table were encountered at site in question
- 3. FOS = 3.0
- 4. Divide & arrange the piles required in three (3) rows equally

<u>Q.4</u>) Design a strip footing to support a brick wall having a thickness of 300mm resisting a working dead load of (DL = 170KN) and working live load of (LL = 135KN). The allowable bearing capacity = 220KPa, $f_{c'} = 25MPa \& f_y = 420MPa$ for both the footing and the wall's concrete. FOS for dead load = 1.4 and 1.7 for the live load. Sketch the footing at the end of the design process. Use Ø16 for main bars and Ø12 for distribution bars

Best of Luck

Assist. Lecturer Ali J. Nouri

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Solution:

<u>Q.1:</u>

Corresponding pressure to 30mm of first Plate = 600KPa Load = A X Pr. = π X 0.3² X 0.25 X 600 = 42.41KN Corresponding pressure to 30mm of second Plate = 300KPa Load = A X Pr. = π X 0.6² X 0.25 X 300 = 84.82KN

Q = Am + Pn

42.41kN = $(\pi/4) X (0.3)^2 X m + (\pi) X (0.3) X n$	(1)
84.82kN = $(\pi/4) \times (0.6)^2 \times m + (\pi) \times (0.6) \times n$	(2)

Solving (1) & (2) yields:

m = -0.320073 kN/m² n = 45.0453 kN/m

Q = Am + Pn

850 = B² X -0.320073 + 4B X 45.0453 -0.320073B² + 180.1812B - 850 = 0

Solving for B yields, B = 4.75m, Say B = 5.0m

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<u>Q.2:</u>

At first layer:

$$\sigma_o'$$
 = 1.5 X 18 + 1.5 X (20 – 9.81) = 42.285kPa

q_{net} = (93750/15 X 25) = 250 KPa

Z	М	Ν	I_{σ} X 4	$\Delta\sigma' = I_{\sigma} \times q_{net}$
0	8	8	1	1 X 250 = 250
1.5	5	8.333	0.996	0.996 X 250 = 249
3	2.5	4.1666	0.974	0.974 X 250 = 243.5

 $\Delta \sigma'_{ave.} = \frac{1}{6} (\Delta \sigma'_t + 4\Delta \sigma'_m + \Delta \sigma'_b) = \frac{1}{6} (250 + 4 \times 249 + 243.5) = 248.25 \text{ kPa}$

$$S_{c} = \frac{C_{S}H_{c}}{1+e_{o}}\log\frac{\sigma_{o}'+\Delta\sigma_{ave.}'}{\sigma_{o}'}$$
$$S_{c} = \left(\frac{0.04\times3}{1+0.72}\log\frac{42.285+248.25}{42.285}\right) \times 1000$$

Total Settlement = 58.4mm

At third layer:

$$\sigma_o' =$$
 1.5 X 18 + 3 X (20 – 9.81) + 3 X (20 – 9.81) + 4.5 X (20 – 9.81) = 133.995kPa

q_{net} = (93750/15 X 25) = 250 KPa

Z	М	Ν	$I_{\sigma} X 4$	$\Delta\sigma' = I_{\sigma} \times q_{net}$
6	1.25	2.083	0.861	0.861 X 250 = 215.25
10.5	0.714	1.190	0.634	0.634 X 250 = 158.5
15	0.5	0.833	0.449	0.449 X 250 = 112.25

$$\Delta \sigma'_{ave.} = \frac{1}{6} (\Delta \sigma'_t + 4\Delta \sigma'_m + \Delta \sigma'_b) = \frac{1}{6} (215.25 + 4 \times 158.5 + 112.25) = 160.25 \text{ kPa}$$

$$S_{c} = \frac{C_{S}H_{c}}{1+e_{o}}\log\frac{\sigma_{o}' + \Delta\sigma_{ave.}'}{\sigma_{o}'}$$
$$S_{c} = \left(\frac{0.04 \times 9}{1+0.72}\log\frac{133.995 + 160.25}{133.995}\right) \times 1000$$

Total Settlement = 71.50mm

Total Primary Consolidation Settlement = 58.4 + 71.5 = 129.9mm

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<u>Q.3:</u>

400 X 400mm Precast Concrete Driven Pile

$$- \mathbf{Q_{f1}} = \alpha C A_f = 1 \times 30 \times 0.40 \times 4 \times 5.0 = 240 \text{kN}$$

- $\mathbf{Q}_{f2} = \sigma_{ave.} \times K \times \tan(S) \times A_f$

 $\sigma_{ave.}$ = 5 X 18 + 4 X 18 = 162kN

Q_{f2} = 162 X 1.5 X tan (0.75 X 20) X (0.4 X 4 X 8) = 833.4kN

- $\mathbf{Q}_{f3} = \sigma_{ave.} \times K \times \tan(S) \times A_f$

 $\sigma_{ave.}$ = 5 X 18 + 8 X 18 + 3.5 X 18 = 297kN

Q_{f3} = 297 X 2 X tan (0.75 X 30) X (0.4 X 4 X 7) = 2755.68KN

- $\mathbf{Q}_{\mathbf{f4}} = \alpha C A_f$

L/B = 10/0.4 = 25,

Case (1),

 $\mathbf{Q}_{f4} = \alpha C A_f = 1.1 \times 100 \times (0.4 \times 4 \times 10) = 1760 \text{kN}$

 $\Sigma Qf = 240 + 833.4 + 2755.68 + 1760 = 5589.08$ kN

 $Q_{ult.} = 144 + 5589.08 = 5733.08$ kN

Q_{all. - comp.} = 5733.08/3 = 1911.03kN

No. of piles required = 21000/1911.03 = 11, use 18 Piles

No. of piles in a row = 18/3 = 6

Spacing between Piles = 3d = 3 X 400mm = 1200mm

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$$E_g = 1 - \emptyset \left[\frac{(n'-1)m + (m-1)n'}{90mn'} \right] = 1 - 18.417 \left[\frac{(6-1) \times 3 + (3-1) \times 6}{90 \times 3 \times 6} \right]$$
$$= 0.69305$$

Pile Group Capacity = 18 X 1911 X 0.69305 = 23840KN > 21000 KN, Ok

Pile Cap Length = 5 X 1.2 + 0.4 + 0.3 = 6.7m Width of Pile Cap = 2 X 1.2 + 0.4 + 0.3 = 3.1m

Check for Block Shear:

 $\sigma_{ave.}$ = 30 X 18 = 540kN

Q_{b.ult.} = (CNc' + qN_{q'}) X Ab = (100 X 9 + 540 X 1) (6.4 + 2.8) X 2 = 26496Kn

- $\mathbf{Q}_{f1} = \alpha C A_f = 1 \times 30 \times (6.4 + 2.8) \times 2 \times 5.0 = 2760$ kN

- $\mathbf{Q}_{f2} = \sigma_{ave} \times K \times \tan(S) \times A_f$

 $\sigma_{ave.}$ = 5 X 18 + 4 X 18 = 162kN

Q_{f2} = 162 X 1.5 X tan (20) X (2 X (6.4 +2.8)) X 8 = 13019kN

- $\mathbf{Q}_{f3} = \sigma_{ave.} \times K \times \tan(S) \times A_f$

 $\sigma_{ave.}$ = 5 X 18 + 8 X 18 + 3.5 X 18 = 297kN

Q_{f3} = 297 X 2 X tan (30) X (6.4 + 2.8) X 2 X 7 = 44171KN

- $\mathbf{Q}_{f4} = \alpha C A_f = 1.0 \times 100 \times (6.4 + 2.8) \times 2 \times 10 = 18400$ kN

 $\Sigma Qf = 2760 + 13019 + 44171 + 18400 = 78350$ kN

Q_{ult.} = 26496 + 78350 = 104846kN

Q_{all. - comp.} = 104846/3 = 34948kN > 21000KN, OK

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<u>Q.4:</u>

$$\begin{aligned} &A_{f} = (DL + LL)/Q_{all.} = (170 + 135)/220 = 1.386m^{2} \\ &B = 1.5m \\ &P_{u} = 1.4DL + 1.7LL = 1.4 \times 170 + 1.7 \times 135 = 467.5KN/m \\ &Q_{u} = P_{u}/A_{f} = 467.5/1.5 = 311.67KPa/m \\ &Vc = \frac{\theta}{6}\sqrt{f_{c}'} = (0.75/6) \times (25^{0.5}) \times 1000 = 625KN/m^{2} \\ &(\frac{B-a}{2} - d) \times L \times q_{u} = L \times d \times V_{c} \\ &(((1.5 - 0.3)/2) - d) \times 1 \times 311.67 = 1 \times d \times 625 \\ &187.002 - 311.67d = 625d \\ &d = 187.002/(311.67+625), d = 0.199m = 199mm \\ &M_{u} = (q_{u} \times l^{2})/2, l = (B/2 - b/4) = (1.5/2) - (0.3/4) = 0.675m \\ &M_{u} = (311.67 \times 0.675^{2}/2) = 71kN.m \\ &R_{u} = M_{u} \times 10^{6/} (\emptyset Bd^{2}) = 71 \times 1\ 000\ 000/(0.9 \times 1000 \times 199^{2}) = 1.992 \\ &m = f_{v}/0.85f_{c'} = 420/(0.85 \times 25) = 19.76 \\ &\rho = \frac{1}{m} (1 - \sqrt{1 - \frac{2mR_{u}}{f_{y}}}) = \frac{1}{19.76} (1 - \sqrt{1 - \frac{2\times 19.76 \times 1.992}{420}}) = 0.0049887 > \rho_{min.} = 0.002 \\ &\rho_{max.} = 0.75 \times \left(\frac{0.85 \times \beta_{1} \times f_{c}'}{f_{y}} \times \frac{600}{600 + f_{y}} \right) = 0.75 \times \left(\frac{0.85 \times 0.85 \times 25}{420} \times \frac{600}{600 + 420} \right) = 0.0253 \end{aligned}$$

 $\rho_{max.} > \rho > \rho_{min.}$

 $A_{s-tension} = 0.0049887 X 1000 X 199 = 992 mm^2/m$

Area of steel bar $Ø16 = 201 \text{mm}^2$

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No. of bars/m = 992/201 = 4.93 bras, spacing = 1000/5 = 200mm Use Ø16@200mmC/C

 $\rho_{sec.} = 0.3 \rho = 0.3 \times 0.0049887 = 0.0015 < \rho_{min.}$, then use $\rho_{min.} = 0.002$

As-secondary = 0.002 X 1500 X 199 = 597mm²/m

Area of steel bar Ø12 = 113 mm²

No. of bars within the width of the strip = 597/113 = 6bras, spacing = 1500/6 = 250mm

Use Ø12@250mmC/C

Total Thickness of the strip footing = 199 + (12+16)/2 + 75 = 288mm, say 300mm

To calculate the development length:

- 1. $L_d = 0.02 \text{ X } A_b \text{ X } (f_y/(f_{c'})^{0.5}) = 0.02 \text{ X } 201 \text{ X } (420/5) = 337.7 \text{mm, or}$
- 2. $L_d = 0.058 \text{ X } d_b \text{ X } f_y = 0.058 \text{ X } 16 \text{ X } 420 = 390 \text{ mm}$, or
- 3. L_d = 300mm

Then $L_d = 390$ mm

Available $L_d = ((1500 - 300)/2) - 75 = 525 \text{mm} > 390 \text{mm}$, OK