

## Module (Course Syllabus)

### Catalogue 2022-2023

College	Erbil Technical Engineering College	
Department	Highway Engineering Department	
Module Name	Soil Mechanic s	
Module Code	SOM603	
Degree	Technical Diploma <input type="checkbox"/> Bachelor <input checked="" type="checkbox"/> High Diploma <input type="checkbox"/> Master <input type="checkbox"/> Ph <input type="checkbox"/>	
Semester	6 <sup>th</sup>	
Qualification		
Scientific Title	Assistant Lecturer	
Credit	6	
Module type	Prerequisite <input type="checkbox"/> Core <input checked="" type="checkbox"/> Assist. <input type="checkbox"/>	
Module type	Core	
Weekly hours	6	
Weekly hours (Theory)	(4)hr Class	(162) hrs Workload
Weekly hours (Practical)	(2)hr Class	
Number of Weeks	12	
Lecturer (Theory)	Bafreen Chalabi Zero	
E-Mail	bafrin.chalabi@epu.edu.iq	
Mobile	0	
Lecturer (Practical)	Ahmed Suad ALi	
Email	ahmed.ali@epu.edu.iq	
Mobile		



		Task	Weight (Marks)	Due Week	Relevant Learning Outcome
<b>Evaluation</b>		Paper Review		Depending on activity given	Each activity will give storm braining and additional knowledge to the subject
	Assignments	Homework	5%		
		Class Activity	2%		
		Report	10%		
		Seminar			
		Essay			
		Project			
	Quiz	8%			
	Lab.	10%			
	Midterm Exam (Theory)	10%			
	Midterm Exam (Pract.)	15%			
	Final Exam(thr)	20%			
	Final Exam(pract.)	20%			
	Total	100%			
<b>Specific learning outcome:</b>	One basic and very important objective of study Soil Mechanics is: The Soil Mechanics lectures will help students to learn and easily recognize of the Soil Mechanics, which it is relate to all of the civil engineering and highway engineering.				
<b>Course References:</b>	<ul style="list-style-type: none"> <li>➤ Principle of Geotechnical Engineering and Solution manual by Braja M.Das Ed-2009</li> <li>➤ Introduction Soil Mechanics by bela Bodo and Colin Jones</li> <li>➤ Experimental Soil Mechanics by Jean-pierree Bardet</li> <li>➤ Soil Mechanics in Engineering Practice by karl Terzaghi, Ralph B. Peck and Gholamr eza Mesri</li> <li>➤ Basic and Applied Soil Mechanics by Gopal Ranjan</li> <li>➤ Limit Analysis and Rheological Approach in Soil Mechanics (CISM International Centre for Mechanical Sciences, 217) (English and French Edition) 1978th Edition, French Edition, by W. Olszak (Editor), L. Suklje (Editor), G. de Josselin de Jong (Contributor), Z. Mroz (Contributor), C. Szymanski (Contributor)</li> <li>➤ Soil Mechanics Laboratory Manual 9th Edition, by Braja Das.</li> </ul>				

Course topics (Theory)	Week	Learning Outcome
Introduction to Soil Mechanics	1	To perform the Engineering soil surveys. -To develop rational soil sampling devices and soil sampling methods. To develop suitable soil testing devices and soil testing methods. - -To collect and classify soils and their physical properties on the basis of fundamental knowledge of soil mechanics.
Weight-Volume Relations	2	Earth materials are three-phase systems. In most applications, the phases include solid particles, water and air. Water and air occupy voids between the solid particles. For soils in particular, the physical relationship between the solid particles. For soils in particular, the physical relationship between these phases must be examined. A mass of soil can be conveniently represented as a block diagram, with each phase shown as a separate block.
Permeability and Seepage	3	Settlement prediction (preloading), Seepage through and beneath earth structures such as earth dams and retaining walls, In designing of filters in which protect hydraulic structure from piping, Discharge of wells and To determine the amount of water that return into shallow and deep excavations during construction of a project.
Stress with in Soil Masses	4	developed the effective stress concept, which became a key concept in modern soil mechanics. Effective stress in soil contributes to its strength and volume change. It also influences the capillary rise, seepage force due to water flow, quicksand (sand boiling), and heaving at the bottom of the excavation. These are discussed in this chapter.
Total and Effective Stresses	6	The total vertical stress acting at a point below the ground surface is due to the weight of everything lying above: soil, water, and surface loading. Total stresses are calculated from the unit weight of the soil.
Principal Stresses	7	As with any other material, the normal stress at a point within a soil mass is generally a function of the orientation of the plane chosen to define the stress. It is meaningless to talk of the normal stress or the shear at a point.
Consolidation of Soil	9	soils layers undergo a certain amount of compression when subjected to any loading condition. This compression is due to deformation of soil particles, relocation of soil particles, expulsion of water or air from the void spaces.
Lateral Earth Pressure	10	Retaining structures such as retaining walls, basement walls, and bulkheads commonly are encountered in foundation engineering as they support slopes of earth masses. Proper design and construction of these structures require a thorough knowledge of the lateral forces that act between the retaining structures and the soil masses being retained. These lateral forces are caused by lateral earth pressure.
Improvement of Soil (Soil Stabilization by Admixtures) and Clay Minerals	12	The following geotechnical design criteria have to be considered during site selection. -Design load and function of the structure. -Type of foundation to be used. -Bearing capacity of subsoil. Clay minerals are very tiny crystalline substances evolved primarily from chemical weathering of certain rock-forming minerals.

Practical Topics	Week	Learning Outcome
Specific gravity of soil test	1	Specific gravity of soil is useful for determining weight volume relationships and It is used in the computation of most of the laboratory test; such as: hydrometer test, consolidation test.
Soil classification test Sieve analysis test Hydrometer test	4	The object of classification is to arrange soils into groups according to certain characteristics and engineering behavior. Particles size, grading consistency and plasticity generally form the criteria for classification and the hydrometer analysis of soil, based on Stokes' law, calculates the size of soil particles from the speed at which they settle out of suspension from a liquid. Results from the test show the grain size distribution for soils finer than the No. 200 (75 $\mu$ m) sieve. However, when combined with a sieve analysis, offer a complete gradation profile of soils containing coarser materials.
Principles of liquid and plastic limits tests	5	This method covers the laboratory determination of the moisture content of a soil as a percentage of its oven-dried weight. The method may be applied to fine, medium and coarse-grained soils for particle sizes from 2 mm to > 10 mm.
Permeability test	6	The purpose of this test is to determine the permeability (hydraulic conductivity) of a sandy soil by the constant head test method. There are two general types of permeability test method that are routinely performed in the laboratory: 1. The constant head test method, and the falling head test method. The constant head test method is used for permeable soils. 2. The falling head test is mainly used for less permeable soils.
Soil compaction test , Standard Proctors compaction test	7	Laboratory compaction tests are used to determine the relation between water content and dry unit weight and to find the maximum dry unit weight and optimum water content.
Sand cone Method test	8	Find field density in highway or road construction.
CBR test	9	CBR is the ratio expressed in percentage of force per unit area required to penetrate a soil mass with a standard circular plunger of 50 mm diameter at the rate of 1.25 mm/min to that required for corresponding penetration in a standard material. The ratio is usually determined for penetration of 2.5 and 5 mm. When the ratio at 5 mm is consistently higher than that at 2.5 mm, the ratio at 5 mm is used.
Shear strength of soil test • Unconfined compression soil • Triaxial compression test	11	The purpose of this laboratory is to determine the unconfined compressive strength of a cohesive soil sample. We will measure this with the unconfined compression test, which is an unconsolidated undrained (UU or Q-type) test where the lateral confining pressure is equal to zero (atmospheric pressure). The tri-axial shear test is most versatile of all the shear test testing methods for getting shear strength of soil i.e. Cohesion (C) and Angle of Internal Friction ( $\phi$ ), though it is bit complicated. This test can measure the total as well as effective stress parameters both. These two parameters are required for design of slopes, calculation of bearing capacity of any strata, calculation of consolidation parameters and in many other analyses.

Consolidation test	12	Consolidation test is used to determine the rate and magnitude of soil consolidation when the soil is restrained laterally and loaded axially. The Consolidation test is also referred to as Standard oedometer test or One-dimensional compression test. This test is carried out on saturated soil specimens, especially in cohesive soils.
--------------------	----	---

## Questions Example Design

All questions are numerical and problem solving types. An example of a question paper and its solutions are attached at the end of this file.

**Extra notes:** Students can use internet for more explanation and getting extra examples.

## External Evaluator

I hereby confirm that all syllabuses given in the attached course modules is sufficient to cover required subjects, areas and titles needed for students regarding the study year.

**Ahmed Suad Ali:**



Head of QA/QC committee + Senior scientific committee member of Highway Engineering Department/ 21-22

## Sample of Exam

Ministry of Higher Education  
& Scientific Research  
Erbil Polytechnic University  
Erbil Technical Engineering College  
Civil Engineering Department  
Note:



2020 - 2021  
Final Examination

Class: Third  
Subject: Soil Mechanics  
Time : 3 hrs.  
Date : /2/2021  
Code: HE301  
2<sup>nd</sup> Attempt

(Theoretical part)

### Question 1

25 Marks

Define the following mention below:

(Flow Lines, Plastic limit, Rock, Specific surface, Consolidation)

### Question 2

25 Marks

saturated soil stratum 5m thick lies above an impervious stratum and below a pervious stratum. It has a compression index  $C_c = 0.25$  and a coefficient of permeability  $= 3.2 \times 10^{-5}$  cm/sec; its void ratio at a stress of  $1.5 \text{ kg/cm}^2$  is 1.9, compute;

1. The change in void ratio due to increase of stress to  $2 \text{ kg/cm}^2$ .
2. settlement of the soil stratum due to the above increase in stress.
3. time (in min) for 50% consolidation. (Given  $T_v = 0.2$ )

### Question 3

25 Marks

the setup shown below; plot to scale elevation head, pressure head, total head, and seepage velocity versus distance along the sample shown in figure-1.

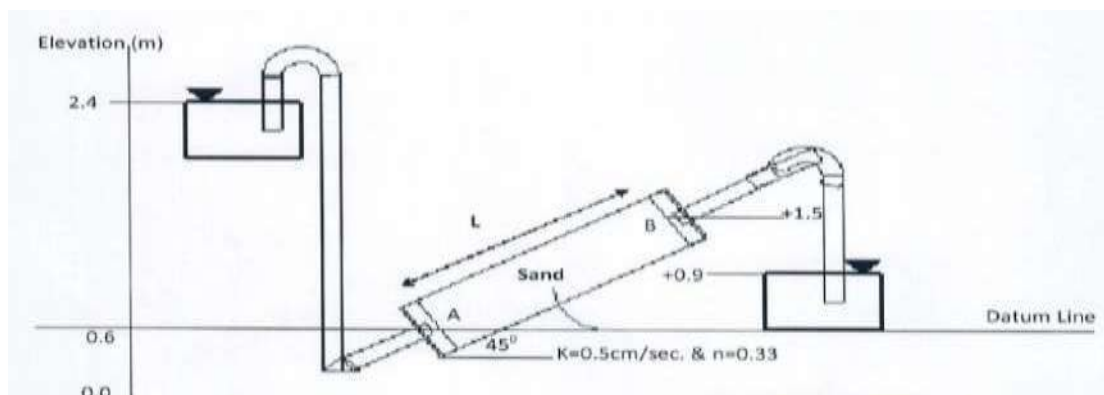


Figure-1

### Question 4

25 Marks

It is required to be excavated from borrow pits for building an embankment as shown in the figure below. The moisture unit weight of the borrow pits is  $18 \text{ kN/m}^3$  and its water content is 8%. Estimate the quantity of earth required to be excavated per meter length of embankment. The dry unit weight required for the embankment is  $15 \text{ kN/m}^3$  with a moisture content of 10%. Assume the specific gravity of solids as 2.67. Also determine the degree of saturation of the embankment soil and the volume of water in the embankment. (hint: Volume of embankment per meter length) shown in figure-2.

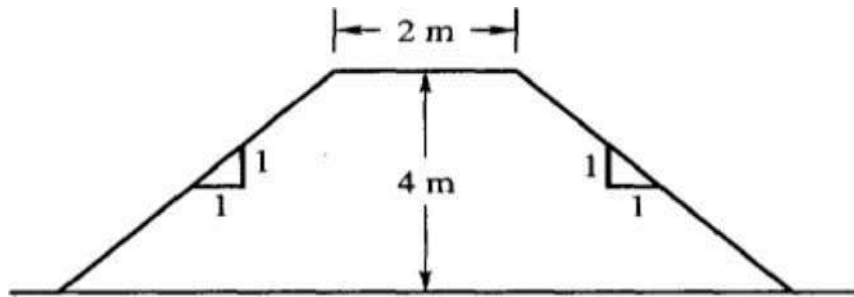


Figure-2

**(Practical part)**

**Question 1**

**10 Marks**

- A. Wirt purpose Liquid, plastic, and specific gravity of soil?
- B. Plot the particle size distributions for each of the soils whose sieve analyses are given below. Find D10, D30, D60 and select type of soil?

Sieve size, (mm)	Mass retained, sample 1	Mass retained, sample 2
37.5	0	15.5
20	0	17
14	0	10
10	0	11
6.30	4.2	33
3.35	3.1	114.5
1.18	55.1	63.3
0.60	26	18.2
0.20	10.4	17
0.063	1	10.5
pan	4.2	2.5

**GOOD LUCK**

**Lecturer Name: Miss. Bafrin Chalabi**

$$n = V_v / V$$

$$G_t = \gamma_t / \gamma_w$$

$$S = (V_w / V_v) * 100$$

$$e = n / (1 - n)$$



$$e = V_v/V_s$$

$$n = e / 1 + e$$

$$W.C = (W_w/W_s) * 100$$

$$\gamma_t = \gamma_d (W.C + 1)$$

$$A_v = (V_a/V_t) * 100$$

$$\gamma_d = (G_s / 1 + e) \gamma_w$$

$$\gamma_t = W_t/V_t$$

$$h_t = h_p + h_z$$

$$\gamma_d = W_s/V_t$$

$$\gamma_d = G_s \gamma_w (1 - n)$$

$$\gamma_s = W_s/V_s$$

$$S_e = G_s m$$

$$\gamma_{sub} = \gamma_{sat} - \gamma_w$$

$$W.c = e S / G_s$$

$$G_s = \gamma_s / \gamma_w$$

$$A = n (1 - S)$$

$$e = ((1 + W.c) G_s \gamma_w / \gamma_d) - 1$$

$$\gamma_t = \rho * g$$

$$PI = LL - PL$$

$$q = k (\Delta h/L) A$$

$$\sigma' = \sigma - u$$

$$\gamma_{sat} = (G_s + e / 1 + e) \gamma_w$$

$$\gamma_d = (\gamma_w / 1 + W.c.)$$

$$S_c = \Delta e / (1 + e_0) * H_0$$

$$mv = (\Delta e / (1 + e_0)) * 1 / \Delta p$$

$$C_c = \Delta e / \Delta \log p$$

$$C_v = K / (mv * \gamma_w)$$

$$T_v = C_v t / d^2$$

### Question 1

25 Marks

Lines, indicating the direction of seepage down a hydraulic gradient (represent the path of water through a soil, the distance between two flow lines is a flow channel, these lines don't intersect).

At the liquid limit, the plastic limit is defined as the moisture content at which the soil crumbles when it is rolled down to a diameter of one-tenth of an inch.

Clay, is a natural aggregate of mineral connected by strong and permanent coherence forces.

Specific surface, Is the surface area per unit mass or volume.

Consolidation, when a saturated soil of low permeability is subjected to a compression stress, the pore water pressure, will immediately increase due to the low permeability of the soil, there will be a time lag between the load application and extrusion of the water and the compression.

### Question 2

25 Marks

$$C_c = \Delta e / \Delta \log p \quad 0.25 = \Delta e / \log 2/1.5 \quad \Delta e = 0.031$$

$$S_c = \Delta e / (1 + e_0) * H_0 = 0.031 / 1 + 1.9 (5) = 0.054 \text{ m} = 5.4 \text{ cm}$$

$$Mv = (\Delta e / (1 + e_0)) * 1 / \Delta p \quad mv = 0.0215 \text{ cm}^2/\text{kg}$$

$$Cv = K / mv * pw = 3.2 * 10 / 0.0215 * (1/1000) * 1 = 14.85 \text{ cm}^2/\text{sec}$$

$$14.85 * 60 = 891.317 \text{ cm}^2/\text{min}$$

$$Tv = Cv t / d^2 = 0.2 * 500 / 891.317 = 56 \text{ min} = t$$

### Question 3

25 Marks

$$\sin 45 = 1.5 - 0.6 / L, \quad L = 1.272 \text{ m}$$

Point	Elevation head $h_e$ (m)	Pressure head $h_p$ (m)	Total head $h_t = h_e + h_p$ (m)	Approach velocity $v$ (cm/sec)
A	0	1.8	1.8	0.589
B	0.9	-0.6	0.3	0.589

$$i = \Delta h / L = 1.8 - 0.3 / 1.272 = 1.178$$

$$v = K i = 1.1785 * 0.5 = 0.58925 \text{ cm/sec}$$

$$v = v / n = 0.589 / 0.33 = 1.785 \text{ cm/sec}$$

### Effective stress in soil with fluid flow

Elevation, m	$v$ (KN/m <sup>2</sup> ) $\delta\Delta$	$v$ (KN/m <sup>2</sup> ) $\delta$	$U$ (KN/m <sup>2</sup> )	$\sigma_v'$ (KN/m <sup>2</sup> )
0.9		0	0	0
	$0.3 * \gamma_w = 2.94$			
0.6		2.94	$0.3 * \gamma_w = 2.94$	0
	$0.6 * \gamma_t = 12.54$			
0		15.48	$1.5 * \gamma_w = 14.72$	0.76

### Question 4

25 Marks

#### A. Embankment

$$V_t = \frac{1}{2} * 4 * 4 + \frac{1}{2} * 4 * 4 + 4 * 2 = 24 \text{ cm}^3$$

$$\gamma_d = (G_s / 1 + e) \gamma_w = 15 = 2.67 * 9.81 / 1 + e \quad e = 0.746$$

$$e = v_t - v_s / v_s = 0.746 = 24 - v_s / v_s \quad v_s = 13.74 \text{ m}^3/\text{m}$$

$$\text{borrow bit : } \gamma_d = (\gamma_w / 1 + W.c.) = 18 / 1 + 0.08 = 16.67 \text{ kn/m}^3$$

$$\gamma_d = (G_s / 1 + e) \gamma_w = e = 0.57$$

$$e = v_t - v_s / v_s = 0.57 = v_t - 13.74 / 13.74 \quad v_t = 21.6 \text{ m}^3/\text{m}$$

$$B. S_e = G_s m = S = 2.67 * 0.1 / 0.746 = 0.358$$

$$C. n = e / 1 + e = 0.746 / 1 + 0.746 = 0.427$$

$$n = V_v / V = 0.427 * 24 = 10.25 \text{ m}^3/\text{m}$$

$$S = (V_w / V_v) * 100 \quad v_w = 0.358 * 10.25 = 3.67 \text{ m}^3/\text{m}$$



